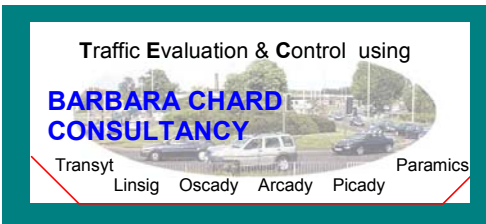


Signal Controlled Roundabouts: Breaking the Rules by

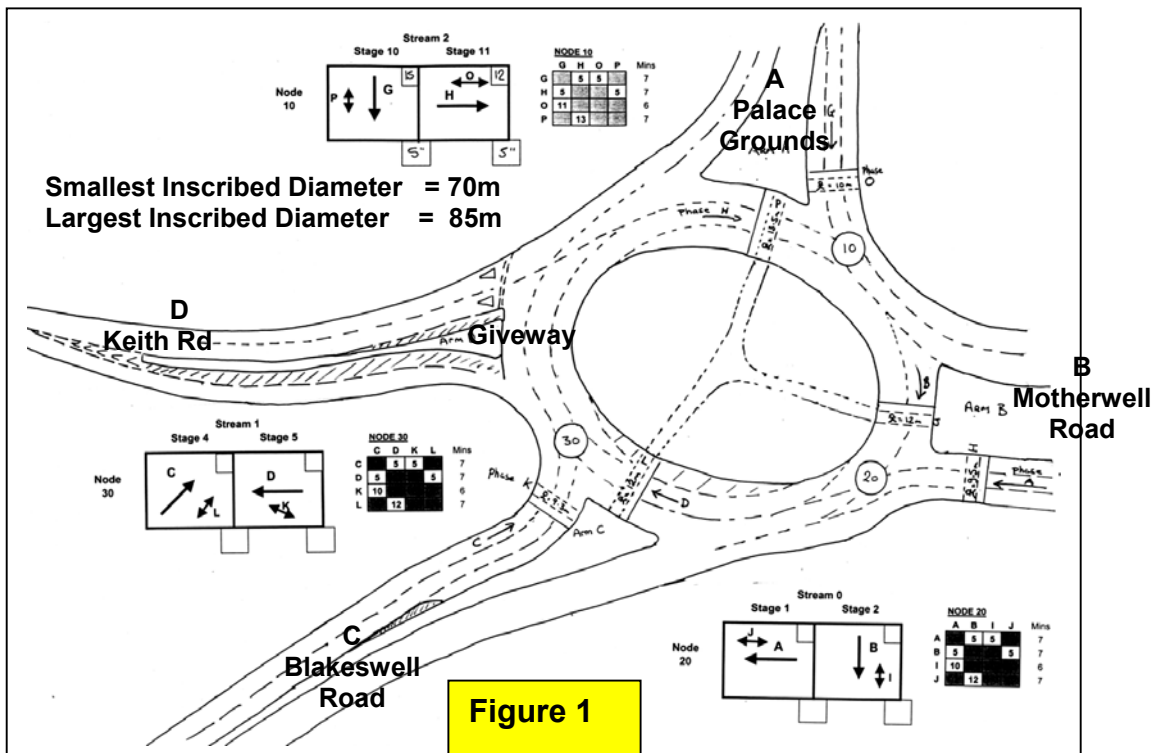
Omero Riccomini (South Lanarkshire Council) and
Barbara Chard (Barbara Chard Consultancy)



*Paper presented at the JCT Traffic signal Symposium,
Nottingham 3-4 July 2002*

1.0 INTRODUCTION

An unusual origin-destination pattern at a small roundabout in Hamilton, Scotland led to an operational solution that contradicts two aspects of the current standard guidance (1,2, 3) for the design of signal controlled roundabouts.



Current design guidance for Signal Controlled Roundabout Design directs you to (see Figures 2 and 3):-

- a) Consider whether one or more approaches may remain uncontrolled.
- b) Seek a cycle time that is as small as possible (*to minimise queuing inside the roundabout*).
- c) Flare the approaches to optimise the available green time (*most of the green time is given to the gyratory links to keep internal queues small and to prevent block-back*). Use LINSAT (4) to ensure flare usage (*as opposed to flare length availability*) is modelled.
- d) Use stop weightings on short internal links carrying newly entered traffic to deter the optimiser from finding timings that would immediately stop newly entered traffic – this for safety. Use the Transyt type 38 excess queue limit facility to deter long internal queues.
- e) Use delay weightings on the approach links to encourage queuing here rather than within the roundabout.
- f) Assemble a Lane/Flow Diagram and use this to properly construct the Link diagram and subsequently assign saturation flows. The new JCT program SIGROUND (5) has been produced to aid this exercise.
- g) Convert all phase control data into stage based format for entry into the Transyt program.
- h) Use the Transyt Graphs (Card type 35) to check platoon progression/s and adjust the Transyt run as necessary to achieve:-
 - Maximim Capacity
 - Satisfactory Progression Trails

Items (b) to (e) above were first advised in the TRL Research Report RR274 , 1990 (3).

Items (a) and (f) to (h) ideology have been developed and significantly refined since 1990, by specialist practitioners, in particular BCC and JCT. Transyt design of signal controlled roundabouts is now taught and demonstrated at both Public and In-House Transyt training courses by Barbara Chard , either for the Barbara Chard Consultancy, BCC (2,3) or for the JCT Consultancy .

2.0 THE HAMILTON ROUNDABOUT

Figure 1 shows the layout of the Hamilton roundabout (*smallest inscribed diameter = 70m, largest inscribed diameter = 85m*). Note that only 3 of the four arms are signal controlled.

Originally all four arms were signal controlled. The roundabout basically did not work! This was dramatically demonstrated one night by bagging the signals at the Keith Rd junction (Arm D).

Note: This roundabout forms the northern limit of a linked signal control system that runs through the town centre of Hamilton. When linking roundabouts to associated networks, the authors advise modelling and finding a best solution first of all, just for the roundabout, then optimising the rest of the network around the already optimised roundabout.

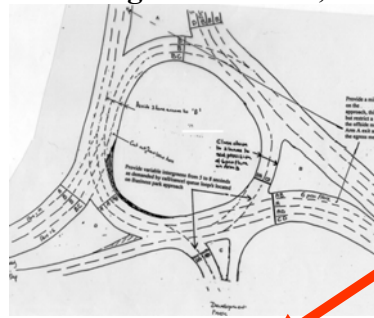
Factors to Consider in Design:-

Consider not signalling an approach arm where:-

- The entry flow is low and there is sufficient stacking room for gap takers at the next stopline in the gyratory
- To avoid a three stage junction within the gyratory
- An upstream platoon exiting at the previous arm provides a natural 'gap'

To signal control 1, 2 3.... Or all Arms?

Some Guidance:

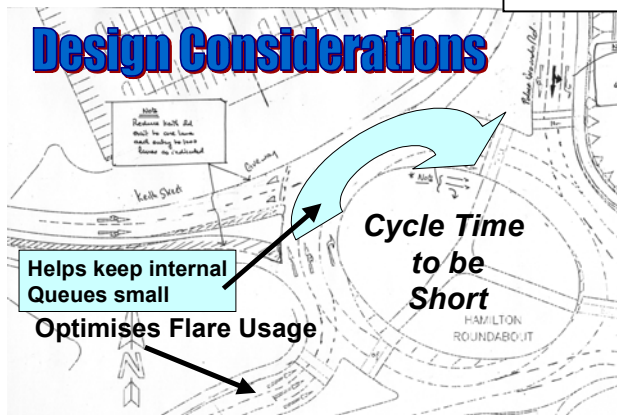


Low entry flow and sufficient stacking room ...

Avoid Three Stage Control at any entry

Did you know that most signal controlled roundabouts work best at **Cycle Times of 60 seconds or less** seconds. Many operating at higher cycle times are not at their most efficient and are the result of allowing Scoot to operate them in the peaks without cycle time length constraint

Design Considerations

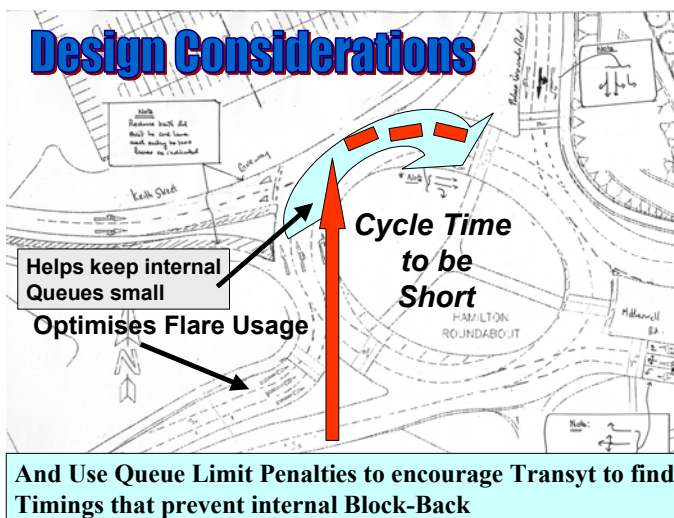


Degree of Saturation

Typical Degrees of Saturation for links in signal controlled gyratories are:-

- Approach links (Up to 95%) saturated
- Gyratory links (60 - 85%) saturated

Design Considerations

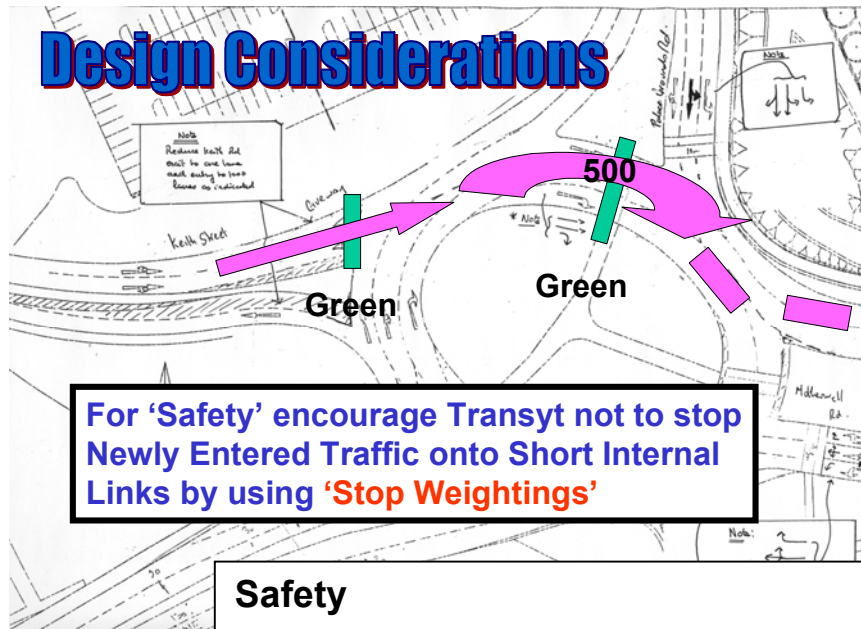


Blocking Back

When modelling signal controlled gyratories it is important to ensure that any spare capacity is given to the circulating links rather than the approach links. This is achieved by a combination of applying low delay and zero stop weightings on the approach links and queue limit penalties on the circulating links.

Figure 2

Design Considerations



Safety

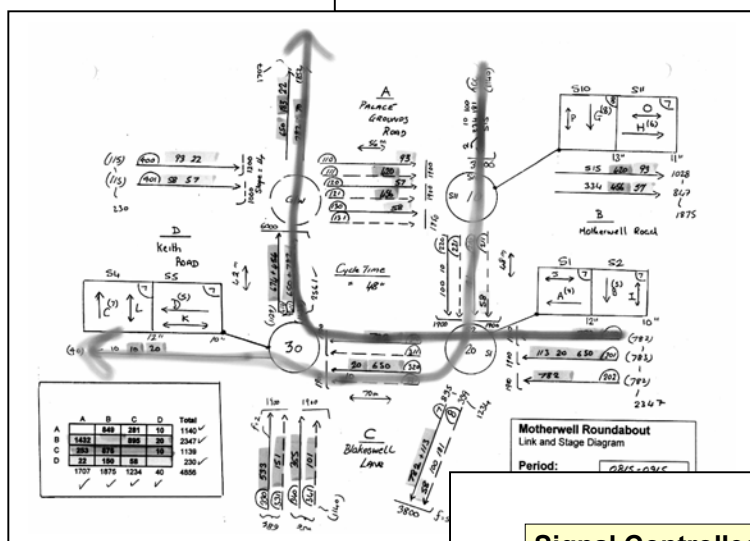
You need to dissuade the optimiser from stopping newly entered traffic onto short gyratory links. To do this:-

- Model newly entered traffic using a separate link; and
- Apply a stop weighting of say 500 on this link.

This will oblige the optimiser to try and find timings that permit this traffic to flow relatively uninterrupted into the gyratory and immediately through the next stopline on green

Checking Co-ordination

Remember to request Transyt Graphs to Check Progression around the roundabout is satisfactory / safe!



Signal Controlled Roundabouts Using Transyt

- Cycle Time needs to be short
- Need to Prepare and Work from a Lane Flow Diagram
- Need to use Shared Links
- Need to use Queue Limit Penalties on Internal Links
- Need to use -9999 (stop) 20% (Delay) on Approach Links
- Apply 500 stop weighting to links carrying newly entered traffic
- Need to Check Progression
- Need to 'fine tune' following implementation

DESIGN CHECKLIST

Figure 3

2.1 Traditional Signal Controlled Roundabout Design

The Saturday peak period follows traditional design methodology. The origin-destination matrix for this period is:-

	A	B	C	D	Total
A		1019	621	21	1661
B	1100		547	192	1839
C	342	510		18	870
D	72	153	81		306
	1514	1682	1249	231	4676

The design solution is summarised in Figure 4. The relevant Lane Flow, Link and Stage diagrams, and Transyt Files are given in **Appendix A**.

Figure 4A presents the initial Transyt run, and Figure 4B the fine tuning and adjustment subsequently made on site. Note that newly entered traffic from Palace Grounds Road (Arm A) and from Motherwell Road (Arm B) is not stopped.

2.2 Weekday PM Peak - Breaking the Rules!

The Weekday peak period posed an interesting problem. The origin destination matrix is as follows:-

	A	B	C	D	Total
A		2079	600	10	2689
B	819		1059	18	1896
C	274	501		10	785
D	62	189	100		351
	1155	2769	1759	38	5721

Note the massive increase in the left-turn movement from the north (Arm A, movement A to B).

This large movement is not matched anywhere else in the roundabout. Accordingly, progression imbalance is inevitable. Moreover, a considerably larger cycle time is wanted at this junction over that necessary at the other two junctions.

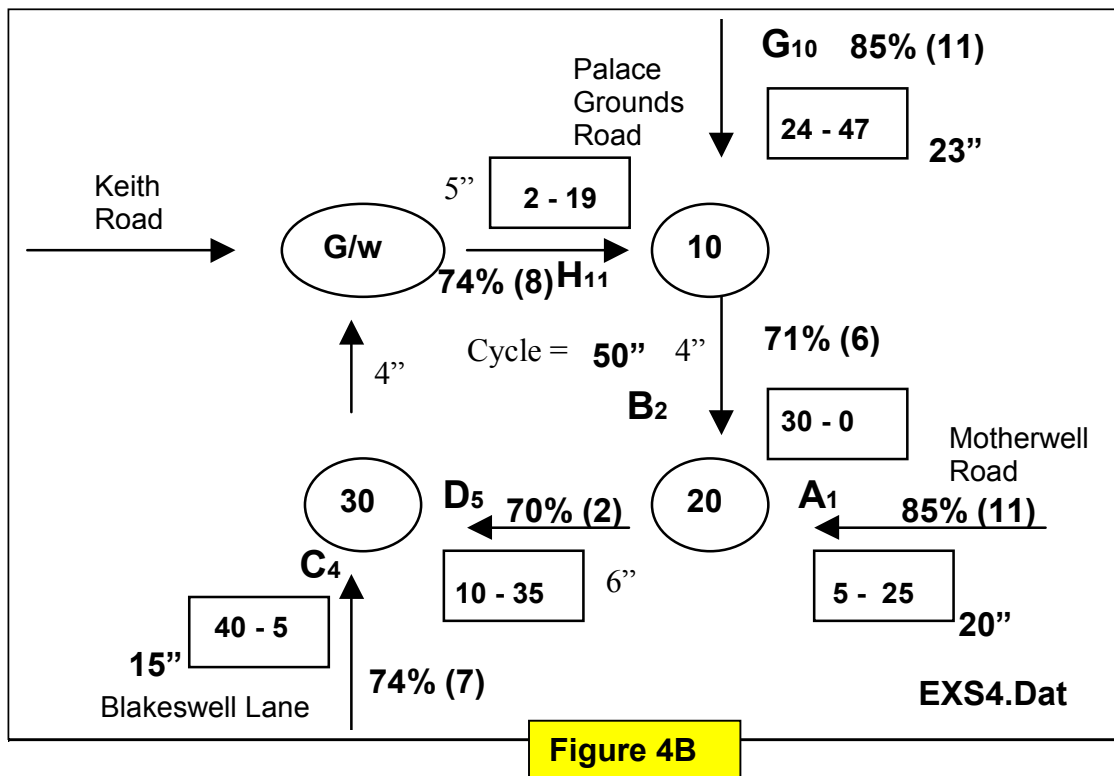
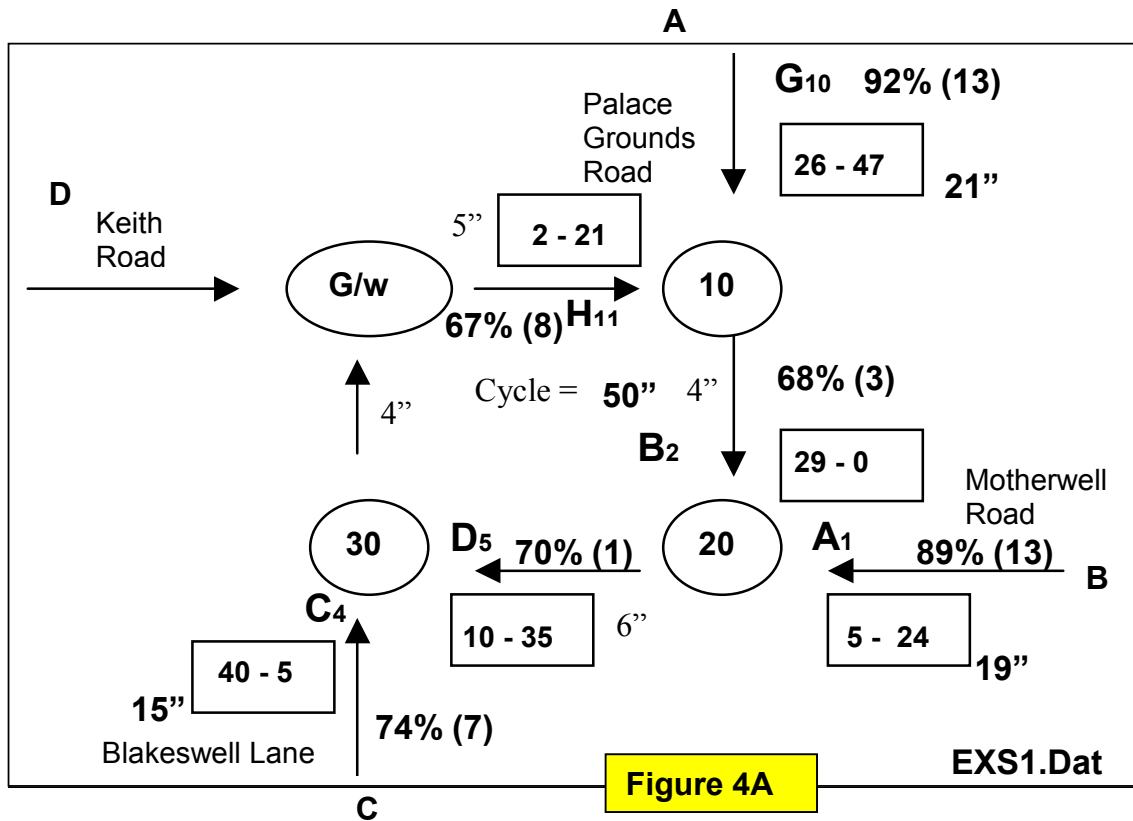
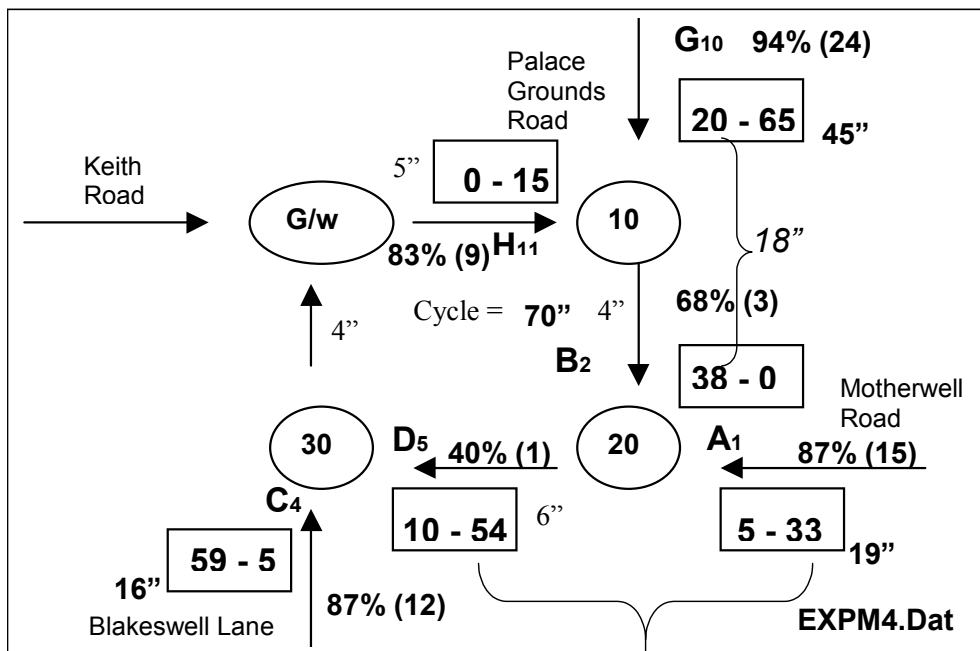
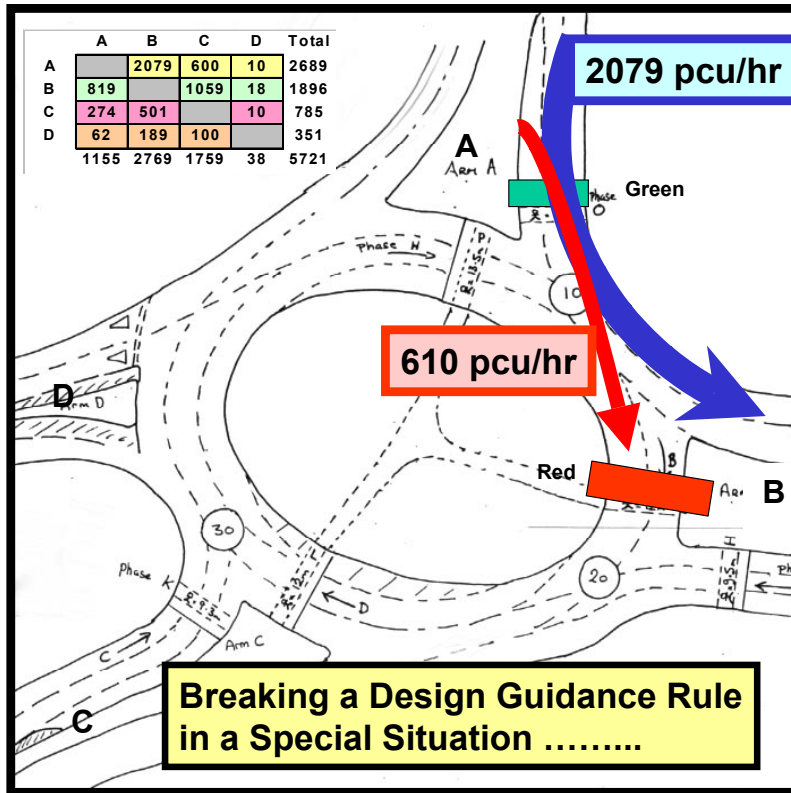


Figure 4 Traditional Signal Controlled Design

An operational solution was found by breaking rule (d) in section 1, i.e. by allowing the newly entering traffic (A – C and A – D) to proceed onto a red light at the first stopline. Only in this way could the necessary large green time be given to approach A. The optimum timings are summarised below:-

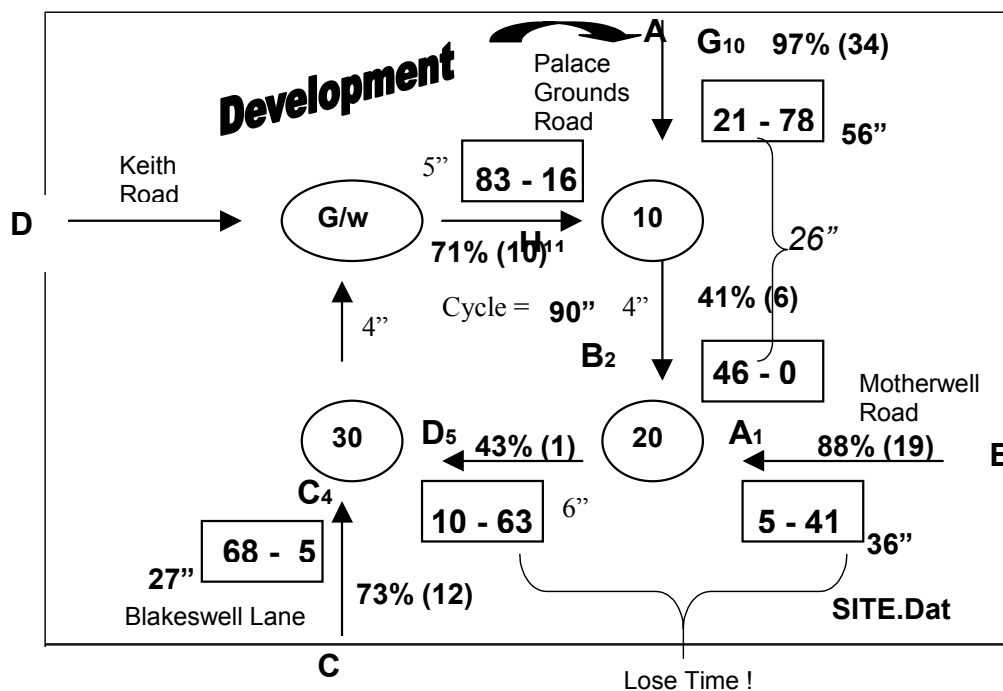


The relevant Lane Flow , Link and Stage diagrams, and Transyt Files for file EXPM4 are given in Appendix B.

Currently, the weekday pm peak period runs a whole town centre plan at 90" cycle. Thus currently on site, this roundabout is 'breaking another guidance rule' by running at a higher than recommended, cycle time. These timings are illustrated below and in Appendix B. Although the previously massive queues on the northern approach are now much reduced, the 90" cycle Plan is exceedingly 'uncomfortable and untidy' to view. Moreover, the wait on red for newly entered straight-ahead traffic from Arm A is uncomfortably long and poor queuing behaviour and lane usage from approach A leads to a lower than expected sat flow from this approach until the straight-ahead gyratory stopline goes green.

However, that said, drivers have adapted very quickly to the unusual progression pattern in the pm peak period and appear unconcerned that this pattern is markedly different from the much more traditional pattern present during other periods in the day.

Due to new development traffic expected shortly, the linking of this roundabout with the rest of the network may be abandoned in the weekday peak period and the 90" plan replaced with the 70" weekday pm plan presented above (EXPM4.dat). This will be aimed at achieving 'sweeter progression' for the B to A movement.



IN SUMMARY

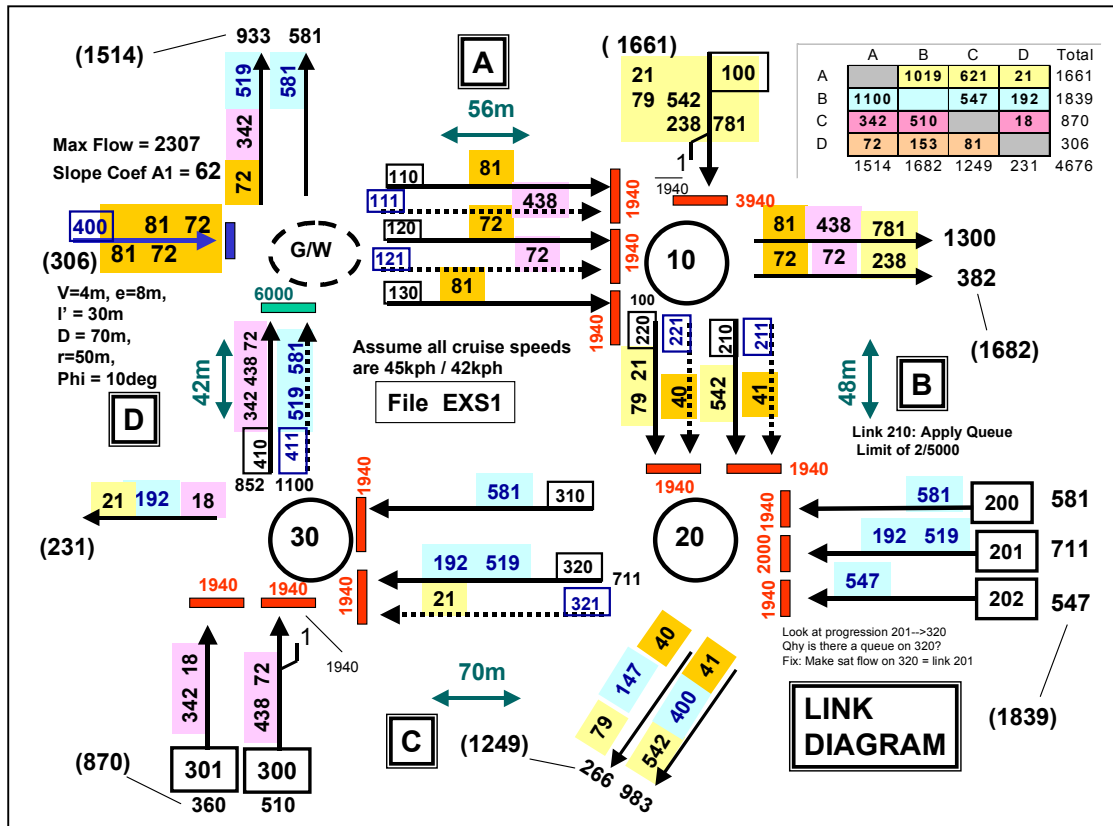
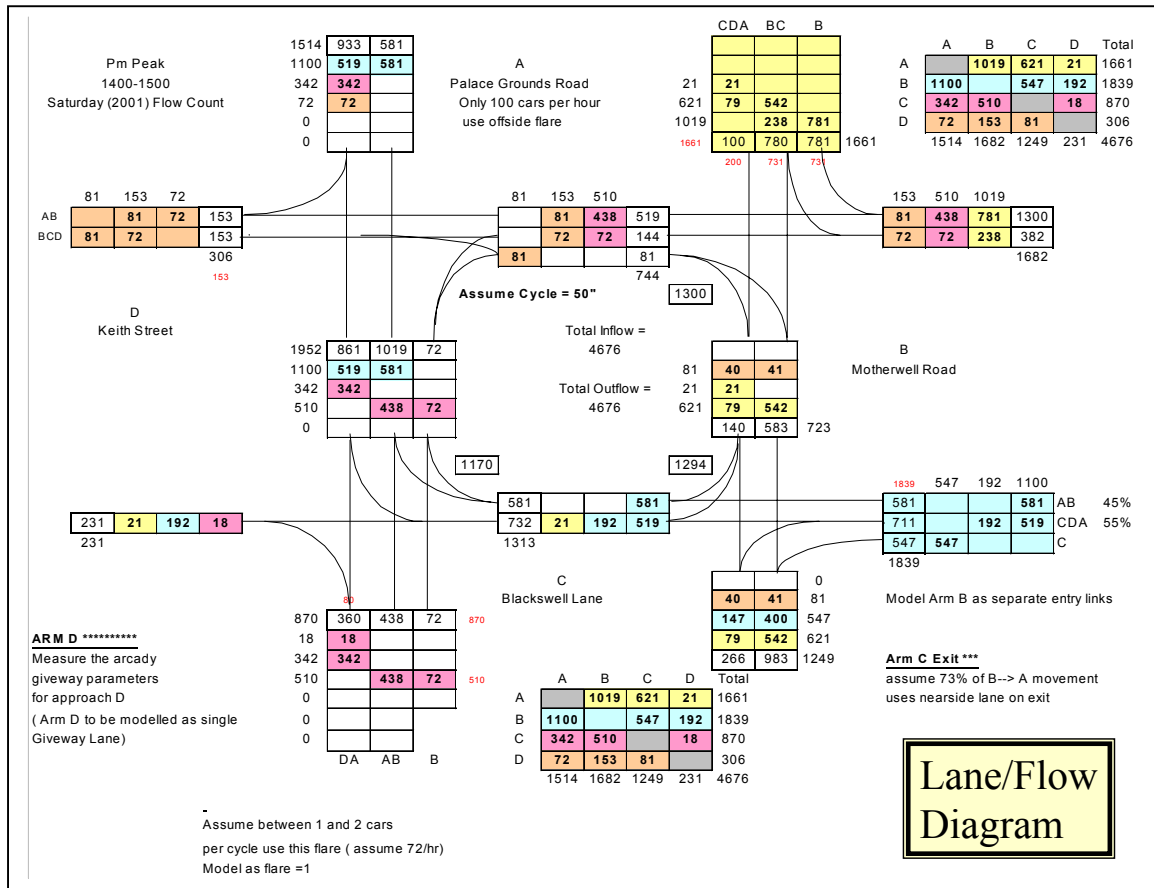
The current guidelines for designing signal controlled roundabouts are generally sound. However, a situation can arise where due to a large imbalance in the origin-destination flows, innovative operational techniques may need to be explored and trailed if gross over-capacity on one approach is to be avoided. The authors have presented one such example in this paper. To date, the unusual progression pattern in the weekday pm peak period has not presented any safety problems. Please note that all innovative and/or unusual applications of signal control need to be monitored closely and/or adjusted in the first few weeks after implementation.

References

1. **Basic Transyt Training** - Comprehensive Design Guidance Notes for Beginners attending a BCC or JCT Transyt/TranEd Training course (2 or 3 day). Contact BCC on 01209 842181 or JCT on 01522 754681.
2. **Advanced Transtt Training** Comprehensive step-by-step guidance for building Lane/Flow Diagrams, Lane Flow balancing, assembly of correct Link structure diagrams, platoon assignment, and running and interpreting Transyt for two signal controlled roundabouts is presented. Other advanced features, including Phase to Stage data conversion and translation of Transyt output data to CLF plan format and are also covered. Comprehensive support notes on the course material are provided as a future reference guide at all BCC or JCT Advanced Transyt Training course/s (2 days). Contact BCC on 01209 842181 or JCT on 01522 754681
3. **TRL Report RR274** – The Use of Transyt at Signal Controlled Roundabouts by Lines C J and M R Crabtree (1990). Transport and Road Research laboratory , Crowthorne Berkshire Tel 01344 773131.
4. **LINSAT** JCT Flare program available FREE off the Internet. Contact JCT on 01522 754681
5. **SIGROUND** JCT Lane/Flow Program. Contact JCT on 01522 754681.

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Appendix A



1	50	50	60	2	3	1	1	0	0	1	2	0	0	1290	235
2	10	20	30												
7	110	111													
7	120	121													
7	210	211													
7	220	221													
7	320	321													
7	410	411													
12	10	0	20	0	17										
12	20	0	19	0	16										
12	30	0	19	0	16										
30	400	410	0	0	62	0	0	0	0	0	200	0	2307		
31	100	10	1	5	2	0	0	0	0	0	200-9999	3940	20		
31	110	10	2	5	1	0	0	0	0	0	56	0	1940		
31	111	0	0	0	0	0	0	0	0	0	56				
31	120	10	2	5	1	0	0	0	0	0	56	0	1940		
31	121	0	0	0	0	0	0	0	0	0	56				
31	130	10	2	5	1	0	0	0	0	0	56	0	1940		
31	200	20	1	5	2	0	0	0	0	0	200-9999	1940	20		
31	201	20	1	5	2	0	0	0	0	0	200-9999	2000	20		
31	202	20	1	5	2	0	0	0	0	0	200-9999	1940	20		
31	210	20	2	5	1	0	0	0	0	0	48	0	1940		
31	211	0	0	0	0	0	0	0	0	0	48				
31	220	20	2	5	1	0	0	0	0	0	48	0	1940		
31	221	0	0	0	0	0	0	0	0	0	48				
31	300	30	1	5	2	0	0	0	0	0	200-9999	1940	20		
31	301	30	1	5	2	0	0	0	0	0	200-9999	1940	20		
31	310	30	2	5	1	0	0	0	0	0	70	500	1940		
31	320	30	2	5	1	0	0	0	0	0	70	500	2000		
31	321	0	0	0	0	0	0	0	0	0	70				
31	410	0	0	0	0	0	0	0	0	0	42	0	6000		
31	411	0	0	0	0	0	0	0	0	0	42				
32	100	1661	0	0	0	45									
32	110	81	0	400	81	45									
32	111	438	0	410	438	42									
32	120	72	0	400	72	45									
32	121	72	0	410	72	42									
32	130	81	0	400	81	45									
32	200	581	0	0	0	45									
32	201	711	0	0	0	45									
32	202	547	0	0	0	45									
32	210	542	0	100	542	45									
32	211	41	0	130	41	42									
32	220	100	0	100	100	45									
32	221	40	0	130	40	42									
32	300	510	0	0	0	45									
32	301	360	0	0	0	45									
32	310	581	0	200	581	45									
32	320	711	0	201	711	45									
32	321	21	0	220	21	42									
32	400	306	0	0	0	45									
32	410	852	0	301	342	45	300	510	45						
32	411	1100	0	310	581	42	320	519	42						
33	100	1940	1												
33	300	1940	1												
35	100	200	210	310	300	0	111								
38	210	2	5000												

Saturday Peak

Zero on call Stage 1

Node	C1	C2
10	21	47
20	0	24
20	35	5

0 50 SECOND CYCLE 50 STEPS

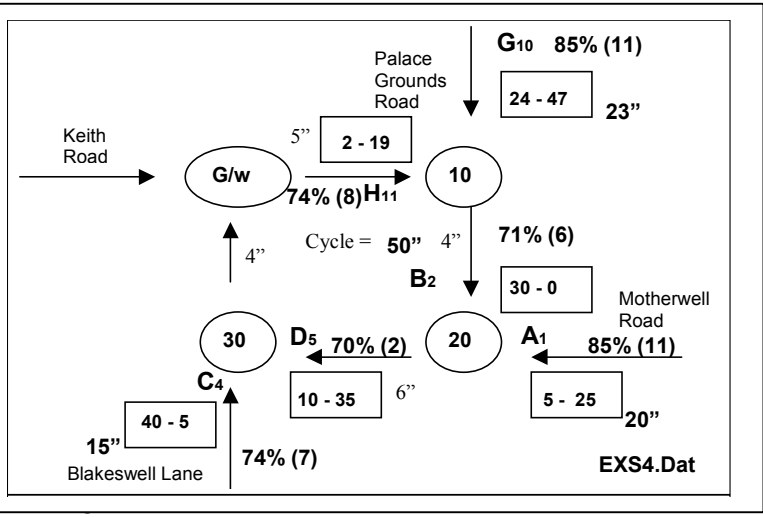
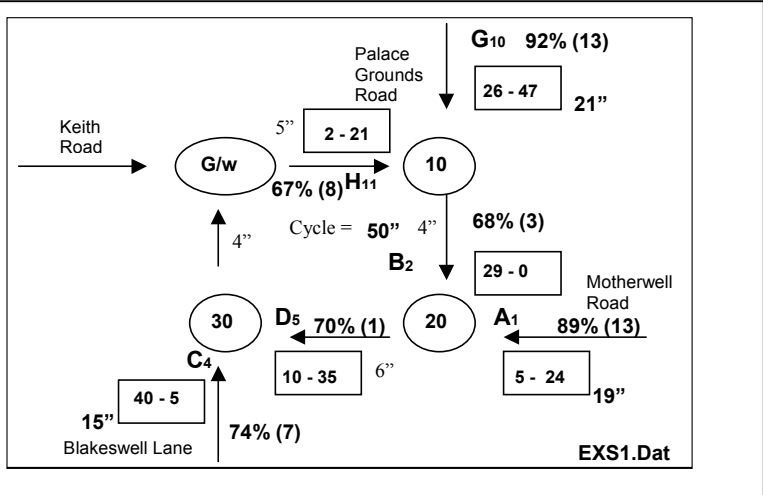
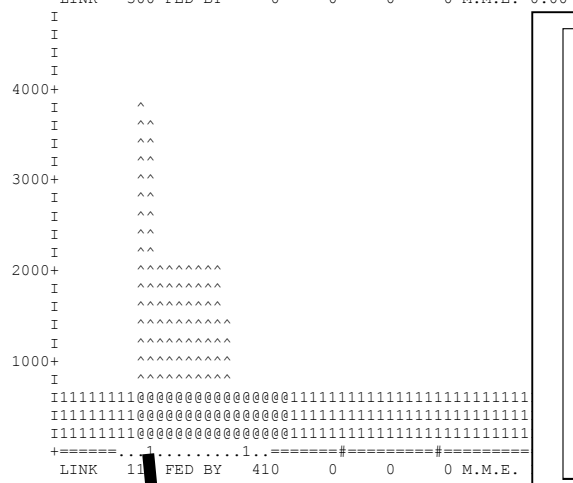
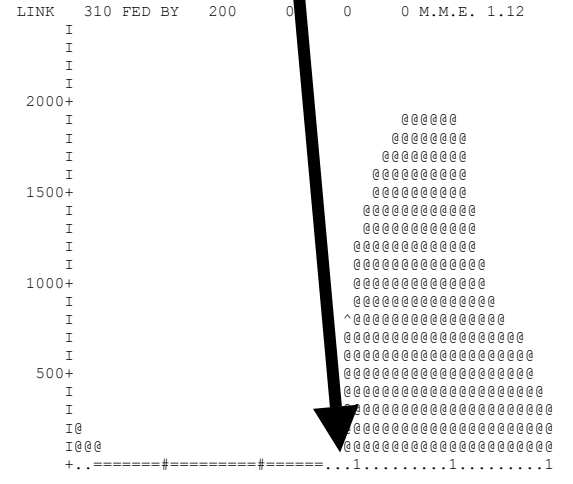
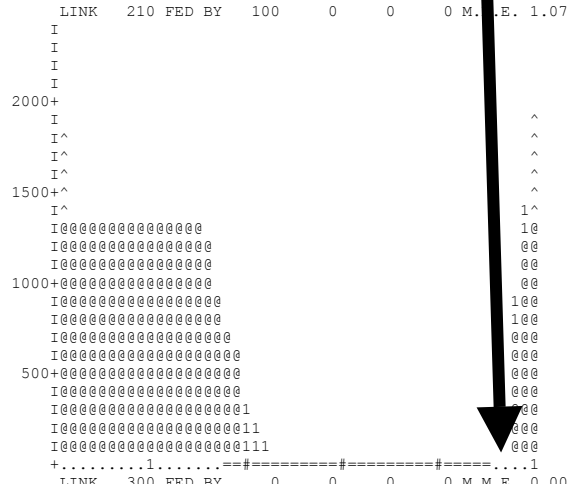
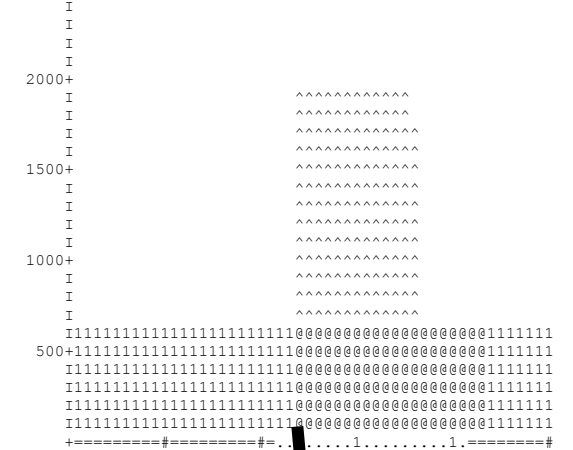
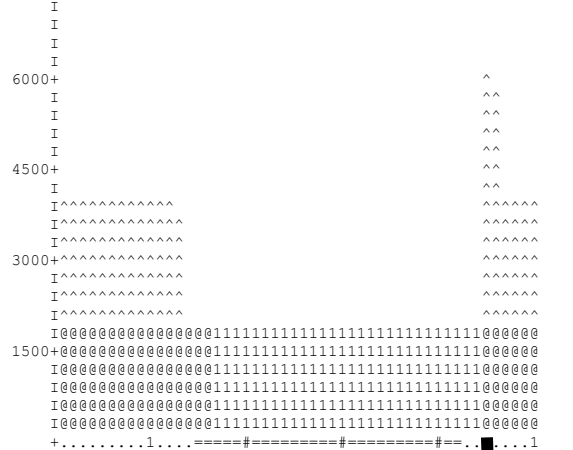
OFINAL SETTINGS OBTAINED WITH INCREMENTS :- 7 20 -1 7 20 1 -1 1

- (SECONDS)

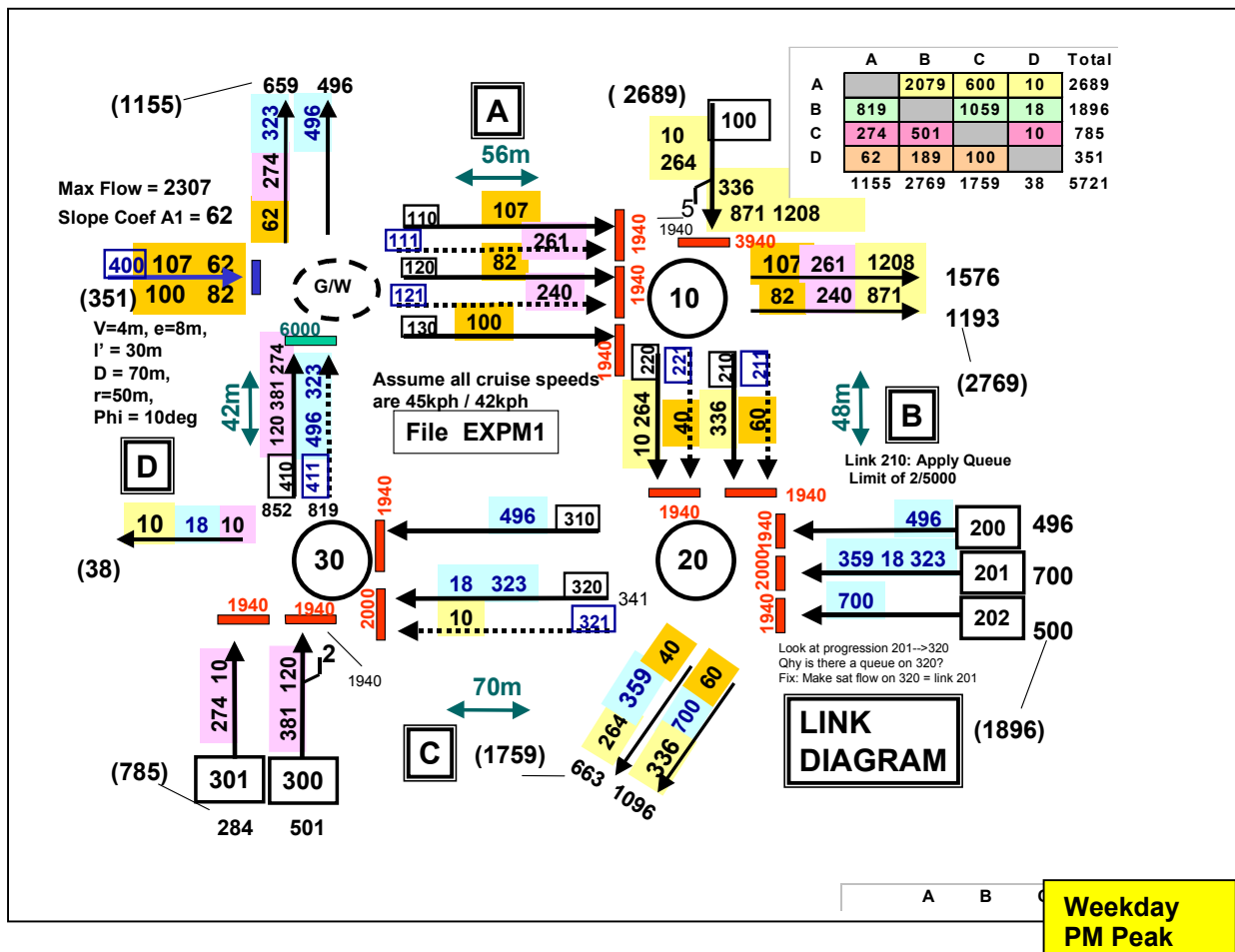
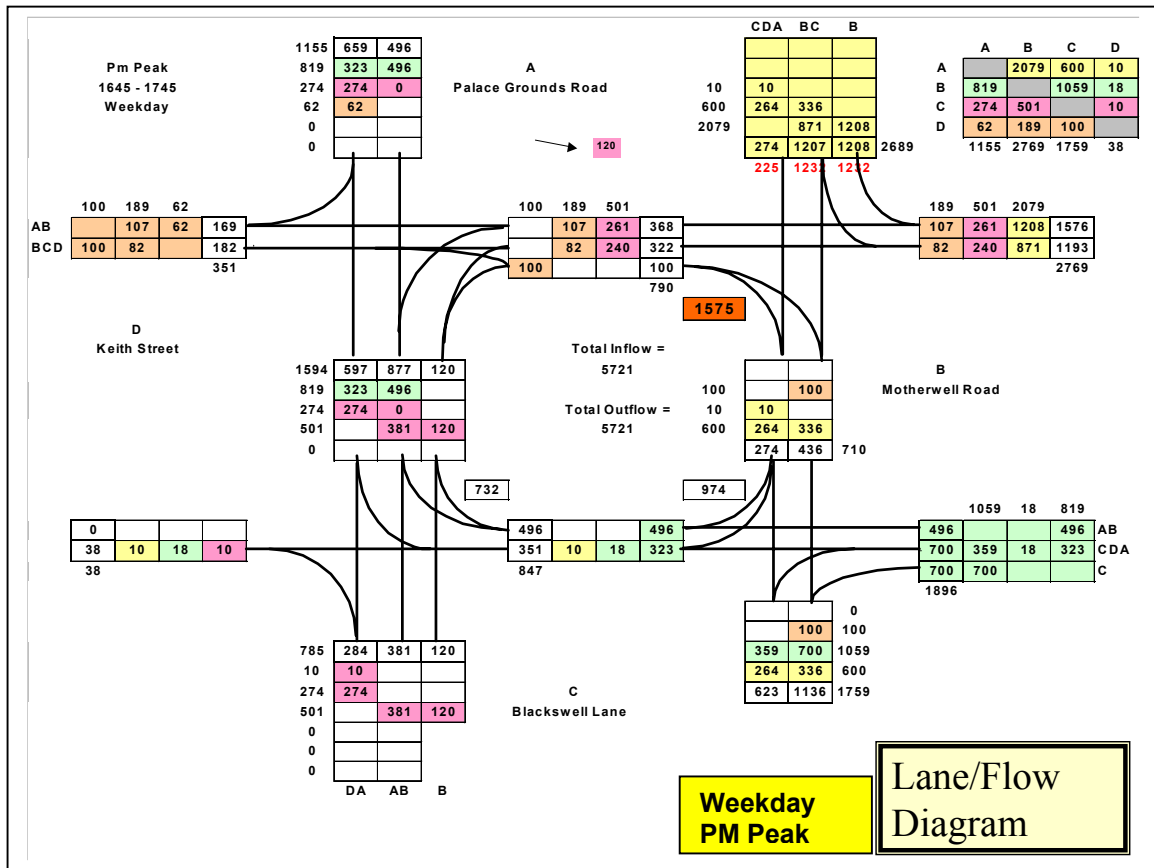
0	NODE NO	NUMBER OF STAGES	STAGE 1	STAGE 2	STAGE 3	STAGE 4	STAGE 5	STAGE 6	STAGE 7
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10	2	37	13						
20	2	16	40						
30	2	1	21						

0 LINK NUMBER	FLOW INTO LINK	SAT FLOW	DEGREE OF SAT	MEAN PER CRUISE	TIMES PCU DELAY (SEC)	-----DELAY----- UNIFORM (PCU-H/H)	RANDOM+ OVERSAT (PCU-H/H)	COST OF DELAY (\$/H)	----STOPS---- MEAN STOPS /PCU (%)	COST OF STOPS (\$/H)	----QUEUE---- MEAN MAX. (PCU)	AVERAGE EXCESS (PCU)	PERFORMANCE INDEX. WEIGHTED SUM OF () VALUES (\$/H)	EXIT NODE	GREEN START END (SECONDS)	TIMES START 1ST 2ND
100	1661	4103F	92	16	24	5.9 +	5.3 (144.9)*	104	(44.2)	27		29.0	10	42	13	
110	81	1940S	67	4	17	0.2 +	0.2 (5.1)	86	(1.8)	8		6.8	10	18	37	
111	438	110L	67	5	12	0.7 +	0.8 (19.5)	90	(8.8)	8		28.3	10	18	37	
120	72	1940S	19	4	12	0.2 +	0.1 (3.0)	54	(1.0)	1		4.0	10	18	37	
121	72	120L	19	5	5	0.0 +	0.1 (1.2)	40	(0.6)	1		1.8	10	18	37	
130	81	1940	10	4	11	0.2 +	0.1 (3.3)	53	(1.1)	1		4.4	10	18	37	
200	581	1940	75	16	22	2.1 +	1.5 (45.7)*	94	(13.9)	8		9.1	20	21	40	
201	711	2000	89	16	33	2.8 +	3.7 (83.2)*	117	(21.3)	13		16.6	20	21	40	
202	547	1940	70	16	20	1.9 +	1.2 (39.8)*	89	(12.5)	7		8.0	20	21	40	
210	541	1940S	68	4	7	0.1 +	1.0 (14.2)	19	(2.6)	3	(0.0)*	18.2	20	45	16	
211	41	210L	68	4	26	0.2 +	0.1 (3.8)	110	(1.0)	3	+	4.8	20	45	16	
220	100	1940S	16	4	3	0.0 +	0.1 (1.1)	9	(0.2)	1		1.4	20	45	16	
221	40	220L	16	4	21	0.2 +	0.0 (3.0)	102	(0.9)	1		4.0	20	45	16	
300	510	2165F	74	16	24	2.0 +	1.4 (43.8)*	97	(12.6)	7		8.8	30	6	21	
301	360	1940	58	16	21	1.4 +	0.7 (27.2)*	89	(8.1)	5		5.4	30	6	21	
310	581	1940	58	6	4	0.0 +	0.7 (8.9)	8	(1.3)	1		15.1	30	26	1	
320	711	2000S	70	6	6	0.0 +	1.1 (15.0)	12	(2.1)	1		25.5	30	26	1	
321	21	320L	70	6	19	0.1 +	0.0 (1.4)	99	(0.5)	1		1.9	30	26	1	
400	306	2307	27	16	3	0.1 +	0.2 (3.4)	25	(1.9)	1		5.3				
410	852	6000S	33	3	0	0.0 +	0.1 (1.4)	1	(0.2)	0		1.6				
411	1100	410L	33	4	0	0.0 +	0.1 (1.8)	1	(0.2)	0		2.0				
*** f - average saturation flow for flared link ***																
0 TOTAL DISTANCE TRAVELLED	TOTAL TIME SPENT	MEAN JOURNEY SPEED	TOTAL UNIFORM DELAY	TOTAL RANDOM+ DELAY	TOTAL COST OF DELAY	TOTAL COST OF STOPS	PENALTY FOR EXCESS QUEUES	TOTAL PERFORMANCE INDEX								
(PCU-KM/H)	(PCU-H/H)	(KM/H)	(PCU-H/H)	(PCU-H/H)	(PCU-H/H)	(\$/H)	(\$/H)	(\$/H)								
0 1185.5	63.0	18.8	18.1	18.4	(163.0) + (37.7)	+	(1.4)	=	202.0	TOTALS						



Appendix B



File EXPM4: Weekday Peak, Motherwell Roundabout

1	70	35	60	2	3	1	1	0	0	1	2	0	0	1290	235
2	10	20	30												
7	110	111													
7	120	121													
7	210	211													
7	220	221													
7	320	321													
7	410	411													
12	10	0	20	0	17										
12	20	0	19	0	16										
12	30	0	19	0	16										
30	400	410	0	0	62	0	0	0	0	0	200	0	2307		
31	100	10	1	5	2	0	0	0	0	0	200-9999	3940	80		
31	110	10	2	5	1	0	0	0	0	0	56	0	1940		
31	111	0	0	0	0	0	0	0	0	0	56	290			
31	120	10	2	5	1	0	0	0	0	0	56	0	1940		
31	121	0	0	0	0	0	0	0	0	0	56	290			
31	130	10	2	5	1	0	0	0	0	0	56	0	1940		
31	200	20	1	5	2	0	0	0	0	0	200-9999	1940	20		
31	201	20	1	5	2	0	0	0	0	0	200-9999	2000	20		
31	202	20	1	5	2	0	0	0	0	0	200-9999	1940	20		
31	210	20	2	5	1	0	0	0	0	0	48	100	1940		
31	211	0	0	0	0	0	0	0	0	0	48				
31	220	20	2	5	1	0	0	0	0	0	48	100	1940		
31	221	0	0	0	0	0	0	0	0	0	48				
31	300	30	1	5	2	0	0	0	0	0	200-9999	1940	20		
31	301	30	1	5	2	0	0	0	0	0	200-9999	1940	20		
31	310	30	2	5	1	0	0	0	0	0	70	2000	1940		
31	320	30	2	5	1	0	0	0	0	0	70	2000	2000		
31	321	0	0	0	0	0	0	0	0	0	70				
31	410	0	0	0	0	0	0	0	0	0	42	0	6000		
31	411	0	0	0	0	0	0	0	0	0	42				
32	100	2689	0	0	0	45									
32	110	107	0	400	107	45									
32	111	261	0	410	261	42									
32	120	82	0	400	82	45									
32	121	240	0	410	240	42									
32	130	100	0	400	100	45									
32	200	496	0	0	0	45									
32	201	700	0	0	0	45									
32	202	700	0	0	0	45									
32	210	336	0	100	336	45									
32	211	60	0	130	60	42									
32	220	274	0	100	274	45									
32	221	40	0	130	40	42									
32	300	501	0	0	0	45									
32	301	284	0	0	0	45									
32	310	496	0	200	496	45									
32	320	341	0	201	341	45									
32	321	10	0	220	10	42									
32	400	351	0	0	0	45									
32	410	775	0	301	274	45	300	501	45						
32	411	819	0	310	496	42	320	323	42						
33	100	1940	5												
33	300	1940	2												
35	100	200	210	310	300	0	111								
38	210	2	5000												

Zero on call Stage 1 at node 20:-

Node	C1	C2
10	15	65
20	0	33
20	54	5

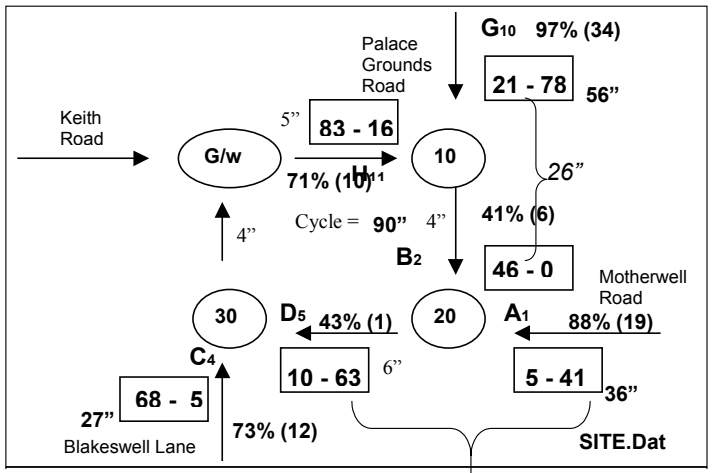
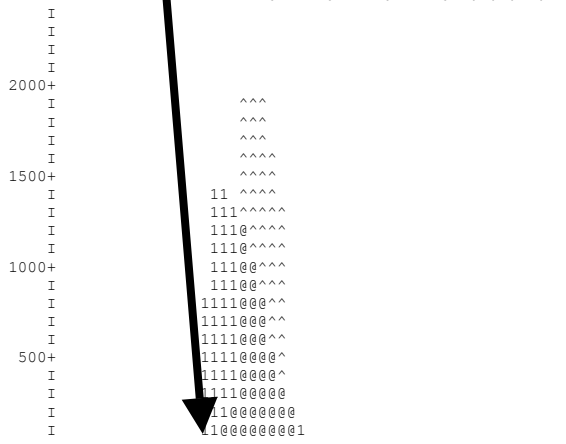
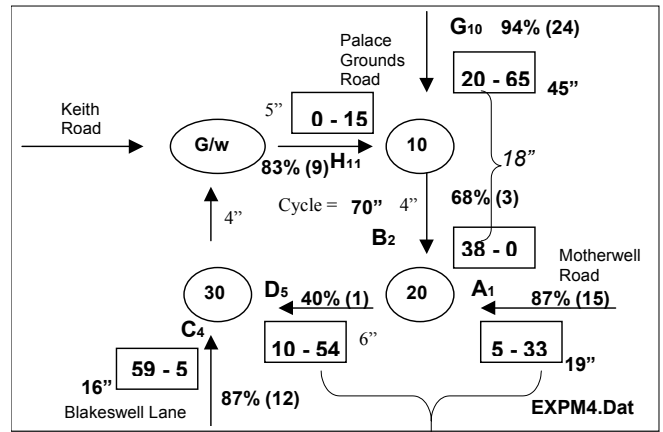
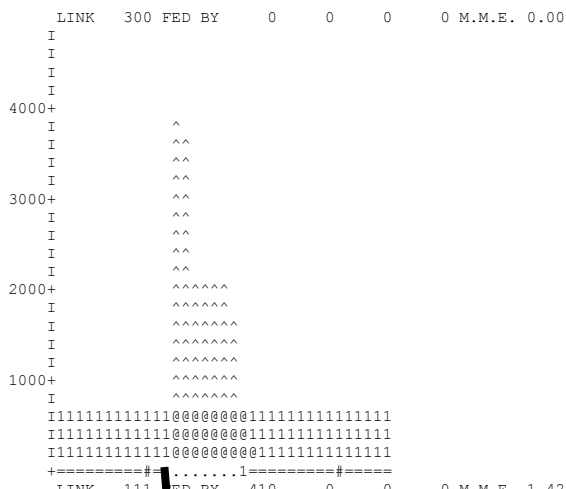
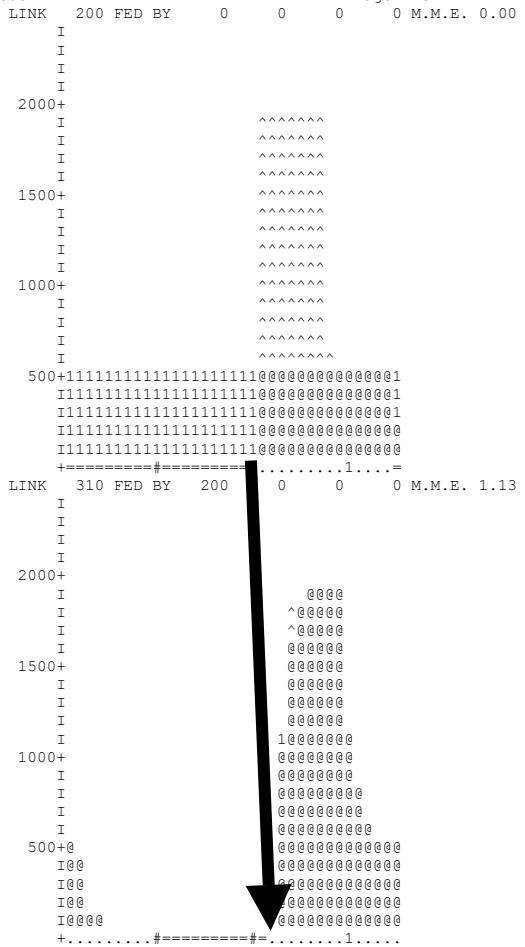
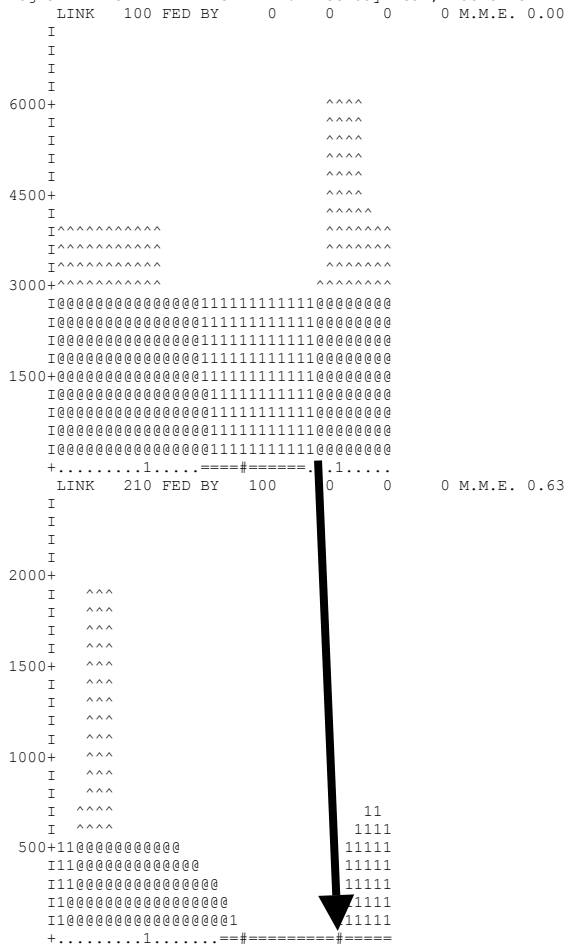
Program TRANSYT File EXPM4: Weekday Peak, Motherwell Roundabout

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0 70 SECOND CYCLE 35 STEPS

0 FINAL SETTINGS OBTAINED WITH INCREMENTS :- 10 28 -1 10 28 1 -1 1

0	LINK	FLOW INTO LINK	SAT FLOW	DEGREE OF SAT	MEAN PER CRUISE	TIMES PCU DELAY	UNIFORM DELAY	RANDOM+ OVERSAT	COST OF DELAY	MEAN STOPS /PCU	COST OF STOPS	QUEUE MAX. (PCU)	AVERAGE EXCESS (PCU)	PERFORMANCE INDEX WEIGHTED SUM OF () VALUES (\$/H)	EXIT NODE	GREEN START	TIMES END	START 1ST	END 2ND	
100	2689	4331f	94	16	20	6.8 + 7.8	(188.5)*	86	(59.3)	53		150.8	10	53	28					
110	107	1940S	83	4	45	0.7 + 0.7	(17.1)	112	(3.1)	9		20.1	10	33	48					
111	260	110L	83	5	28	0.4 + 1.6	(25.7)	100	(5.8)	9		42.5	10	33	48					
120	82	1940S	73	4	37	0.5 + 0.3	(10.8)	98	(2.0)	7		12.8	10	33	48					
121	240	120L	73	5	19	0.3 + 1.0	(16.2)	83	(4.4)	7		29.0	10	33	48					
130	100	1940	22	4	27	0.6 + 0.1	(9.6)	78	(2.0)	2		11.6	10	33	48					
200	496	1940	62	16	22	2.2 + 0.8	(39.0)*	80	(10.1)	8		7.8	20	38	66					
201	700	2000	84	16	32	3.6 + 2.6	(80.1)*	101	(18.0)	14		16.0	20	38	66					
202	700	1940	87	16	35	3.7 + 3.2	(88.2)*	106	(19.0)	15		17.6	20	38	66					
210	333	1940S	43	4	9	0.5 + 0.3	(10.3)	50	(4.3)	5	(0.4)*	36.7	20	1	33					
211	60	210L	43	4	34	0.5 + 0.1	(7.2)	105	(1.4)	5	+	8.6	20	1	33					
220	274	1940S	34	4	8	0.3 + 0.2	(7.4)	45	(3.2)	4		10.6	20	1	33					
221	40	220L	34	4	33	0.3 + 0.0	(4.7)	104	(0.9)	4		5.6	20	1	33					
300	501	2363f	87	16	47	3.4 + 3.2	(84.2)*	118	(15.1)	12		16.8	30	22	38					
301	284	1940	60	16	33	1.9 + 0.8	(33.7)*	96	(7.0)	6		6.7	30	22	38					
310	496	1940	40	6	2	0.0 + 0.3	(4.3)	4	(0.5)	1		14.2	30	43	17					
320	342	2000S	27	6	2	0.0 + 0.2	(2.4)	3	(0.2)	0		7.1	30	43	17					
321	10	320L	27	6	8	0.0 + 0.0	(0.3)	39	(0.1)	0		0.4	30	43	17					
400	351	2307	26	16	3	0.1 + 0.2	(3.6)	17	(1.6)	2		5.1								
410	775	6000S	27	3	0	0.0 + 0.1	(1.1)	1	(0.1)	0		1.2								
411	820	410L	27	4	0	0.0 + 0.1	(1.2)	1	(0.1)	0		1.3								
*** f - average saturation flow for flared link ***																				
0	TOTAL DISTANCE TRAVELLED	TOTAL TIME SPENT	MEAN JOURNEY SPEED	TOTAL UNIFORM DELAY	TOTAL RANDOM+ OVERSAT	TOTAL COST OF DELAY	TOTAL COST OF STOPS	PENALTY FOR EXCESS QUEUES (\$/H)	TOTAL PERFORMANCE INDEX (\$/H)	TOTALS										
0	1348.8	79.3	17.0	25.8	23.5	(337.6)	(63.0)	(22.1)	422.7	TOTALS										



Lose Time !

+-----#-----...1.....-#-----